GENERAL NOTES & LOAD CRITERIA: 1. ALL WORK SHALL BE DONE IN ACCORDANCE WITH THE VIRGINIA STATEWIDE UNIFORM BUILDING CODE, THE 2012 INTERNATIONAL BUILDING CODE AND THE STANDARDS AND SPECIFICATIONS OF THE COUNTY OF LOUDOUN, VIRGINIA. 2. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO NOTIFY "MISS UTILITY" BY TELEPHONE AT 811 AT LEAST TWO (2) WORKING DAYS BUT NOT MORE THAN TEN (10) WORKING DAYS PRIOR TO COMMENCEMENT OF ANY LAND DISTURBING ACTIVITIES. 3. THE CONTRACTOR SHALL BE LICENSED IN THE COMMONWEALTH OF VIRGINIA IN ACCORDANCE WITH CHAPTER 11 OF TITLE 54.1 OF THE CODE OF VIRGINIA. THE CONTRACTOR SHALL BE REQUIRED TO OBTAIN ALL NECESSARY PERMITS PRIOR TO CONSTRUCTION. IT SHALL ALSO BE THE RESPONSIBILITY OF THE CONTRACTOR TO MAINTAIN WORKMAN'S COMPENSATION DEPARTMENT OF LOUDOUN COUNTY, VIRGINIA. INSURANCE AND CONTRACTORS GENERAL LIABILITY COVERAGE. A CERTIFICATE OF INSURANCE SHALL BE SUBMITTED TO THE ALL WORK SHALL BE PERFORMED IN AN EXPEDITIOUS AND WORKMANLIKE MANNER AND IN ACCORDANCE WITH ALL GENERALLY ACCEPTED TRADE PRACTICES. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO MAINTAIN A CLEAN AND ORDERLY SITE THROUGHOUT THE CONSTRUCTION PROCESS. 5. THESE STRUCTURAL DRAWINGS SHALL BE READ IN CONCERT WITH ALL OTHER CONTRACT DOCUMENTS AND SPECIFICATIONS. 6. EVERY EFFORT HAS BEEN MADE TO VERIFY THE DIMENSIONS AND ENSURE THE ACCURACY OF THESE PLANS. HOWEVER, THE CONTRACTOR SHALL VERIFY DIMENSIONS PRIOR TO THE CONSTRUCTION PROCESS AND IF ANY DISCREPANCIES DO EXIST, THEN THE PROJECT ENGINEER OR THE REPRESENTATIVE OF THE TOWN OF MIDDLEBURG, VIRGINIA SHALL BE NOTIFIED BEFORE THE NECESSARY ADJUSTMENTS ARE MADE. 7. DO NOT SCALE THE DRAWINGS. THE DIMENSIONS INDICATED ON THE PRINTS SHALL CONTROL. **ABBREVIATIONS:** THE TYPICAL DETAILS PROVIDED ON THESE DRAWINGS ARE TO DESCRIBE THE CONSTRUCTION STANDARDS TO BE APPLIED FOR THIS PROJECT. THE DETAILS MAY BE ADJUSTED, AS REQUIRED, TO ACCOUNT FOR VARYING CONDITIONS IN THE FIELD. SIGNIFICANT ADJUSTMENTS MUST BE PRESENTED TO THE PROJECT ENGINEER FOR REVIEW AND APPROVAL PRIOR TO ANY INSTALLATION OR FABRICATION. ALL TYPICAL DETAILS SHALL BE REVIEWED BY THE CONTRACTOR AND APPROPRIATE SUBCONTRACTORS PRIOR TO ANY DETAILING, FABRICATION, AND/OR INSTALLATION. ANY CONFLICTS OR CLARIFICATIONS SHALL BE RESOLVED BEFORE THE COMMENCEMENT OF WORK. 9. IF APPLICABLE, SHOP DRAWINGS OF ALL PRE-ENGINEERED STRUCTURAL COMPONENTS SHALL BE SUBMITTED FOR REVIEW PRIOR TO ANY FABRICATION OR INSTALLATION. 10. COORDINATE SIZE AND INSTALLATION OF ALL OPENINGS, SLEEVES, INSERTS, TIES, EQUIPMENT PADS, EQUIPMENT SUPPORTS, ETC. WITH APPROPRIATE TRADES AND CONTRACT DRAWINGS PRIOR TO INSTALLATION OR FABRICATION. 11. THE CONTRACTOR IS RESPONSIBLE FOR THE IMPLEMENTATION OF ALL O.S.H.A. STANDARDS, SO THAT THE CONSTRUCTION SITE IS SECURE AND THAT THE SAFETY OF THE PUBLIC AND ALL PARTIES INVOLVED IN THE CONSTRUCTION PROCESS IS PRESERVED. 12. ADEQUATE TEMPORARY SUPPORT STRUCTURES, SUCH AS SHEETING, SHORING, AND BRACING, SHALL BE INSTALLED DURING CONSTRUCTION UNTIL THE WIND RESISTING SYSTEMS ARE IN PLACE AND THE SUPPORTING DIAPHRAGMS ARE IN PLACE AND HAVE ATTAINED 75% STRENGTH. SHORING AND BRACING SHALL RESIST FORCES SUCH AS WIND AND UNBALANCED LOADING DUE TO CONSTRUCTION. BRACING SYSTEMS SHALL BE DESIGNED BY A REGISTERED PROFESSIONAL ENGINEER LICENSED TO PRACTICE IN THE COMMONWEALTH OF VIRGINIA. IMPOSED CONSTRUCTION LOADS IN EXCESS OF THE STATED DESIGN LOADS MUST BE APPROVED BY THE STRUCTURAL ENGINEER PRIOR TO PLACING OF SUCH LOADS. 3. IF APPLICABLE, BOTH SIDES OF THE FOUNDATION WALL SHALL BE BACKFILLED SIMULTANEOUSLY SO AS TO PREVENT OVERTURNING OR LATERAL MOVEMENT OF THE WALLS. NO FOUNDATION WALLS SHALL BE BACKFILLED WITHOUT THE FLOOR DIAPHRAGM AND BASEMENT SLAB BEING CONSTRUCTED IN PLACE AND SECURE. B. DESIGN CRITERIA: 1. CODES: (USE LATEST EDITIONS U.N.O.) a. INTERNATIONAL BUILDING CODE (2012). b. BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE (ACI-318) c. AISC MANUAL OF STEEL CONSTRUCTION (AISC-13th EDITION). d. BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES. (ACI 530-08/ASCE 5-08/TMS 402-08) 2. LIVE LOADS: 30.0 PSF (MINIMUM) ROOF LOAD CEILING LOADS: 10.0 PSF (LIMMITED TO 10 PSF) FLOOR LOADS 100.0 PSF (ASSEMBLY AREA) 80.0 PSF (BALCONY ASSEMBLY AREAS)** ** OCCUPANCY LIMITATION FOR LOAD REDUCTION DEAD LOADS: ACTUAL MATERIAL WEIGHTS ROOF DIAPHRAGM: 8.50 PSF 20.0 PSF CEILING LOADS: BALCONY FLOOR DIAPHRAGM: 25.0 PSF MAIN FLOOR DIAPHRAGM: 18.0 PSF 3. WIND LOAD: BASIC WIND SPEED 115 MPH IMPORTANCE FACTORS 1.0 **EXPOSURE** В 4. SNOW LOAD: MEAN GROUND SNOW LOAD 30.0 PSF* SNOW EXPOSURE FACTOR 0.7 SNOW IMPORTANCE FACTOR 1.0 *ADJUST FOR SNOW ACCUMULATION, IF APPLICABLE. 5. EARTHQUAKE LOAD: SHORT PERIOD SPECTRAL RESPONSE TIMOTHY G. PAINTER ACCELERATION (SDS) 0.135 Lic. No. 018260 1 SECOND PERIOD SPECTRAL RESPONSE ACCELERATION (SD1) 0.084 SOIL SITE CLASS D SEISMIC HAZARD EXPOSURE GROUP SEISMIC PERFORMANCE CATEGORY SSIONAL ENG REQUIRED TO COMPLY WITH SECTION 1616.2.1 ONLY

6. FOUNDATION DESIGN WAS ESTABLISHED BASED UPON THE BEST AVAILABLE GEOTECHNICAL INFORMATION OBTAINED ABOUT THE SITE. NO FORMAL SOILS REPORT WAS PREPARED FOR THIS SITE.

7. TO VERIFY COMPLIANCE WITH THE DESIGN RECOMMENDATIONS OF THIS PROJECT, ALL FOOTING EXCAVATIONS SHALL BE INSPECTED BY A LICENSED GEO-TECHNICAL ENGINEER OR CERTIFIED INSPECTOR, PRIOR TO PLACEMENT OF THE CONCRETE. FOR THIS PROJECT A SOIL BEARING CAPACITY OF 2500 PSF WAS USED. IF THE SOIL BEARING CAPACITY IS FOUND TO BE LESS THAN THE ASSUMED DESIGN VALUE, THEN THE FOOTINGS MUST BE REDESIGNED ACCORDINGLY.

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8. A MINIMUM FROST DEPTH OF 24" SHALL BE MAINTAINED FOR THIS PROJECT AS REQUIRED BY THE LOCAL BUILDING

9. THIS OBJECTIVE OF THESE PLANS AND THIS PROJECT IS TO PRESERVE THE EXISTING ASBURY CHURCH FOR FUTURE USE. THE PLANS HAVE BEEN DEVELOPED TO SEAL THE STRUCTURE FROM ANIMALS, PESTS, WATER INFILTRATION, AND EXPOSURE TO THE ELEMENTS. THE STRUCTURE IS TO BE STABILIZED, SEALED, AND CLEANED. ALL DAMAGED MATERIALS, DEBRIS, AND ORGANIC MATERIALS SHALL BE FULLY REMOVED. AFTER CONSTRUCTION, THE STRUCTURE SHALL BE BROOM CLEANED ON ALL LEVELS. THIS PLAN IS NOT TO SERVE AS THE PLAN FOR THE RENOVATION OF THIS SPACE FOR FUTURE USE. IT SHALL ONLY SERVE TO PRESERVE THIS STRUCTURE UNTIL THE COMPLETE RENOVATION CAN BE DESIGNED AND CONSTRUCTED.

0	AT
# ø	NUMBER
Ø	DIAMETER
d (16d)	PENNY (16 Penny Nails)
CMU	CONCRETE MASONRY UNIT
EX.	EXISTING
ML or M=L	MICRO=LAM
O.C.	ON CENTER
PR.	PROPOSED
R or (R)	ROUGH (Rough Cut Member)
SPF #2	SPRUCE-PINE-FIR, GRADE 2
SYP #2	SOUTHERN YELLOW PINE, GRADE 2
TYP. (Typ.)	TYPICAL

ASBURY CHURCH 105 NORTH JAY STREET MIDDLEBURG, VIRGINIA 20117 PRESERVATION AND STABILIZATION PLAN

List Of Structural Drawings

	1987 - BONDO - MOSPON INCHESTO NO SERIESTANO - INCHESTRICAN (1987) - 1987.
SHT. CS:	GENERAL NOTES & LOAD CRITERIA
SHT. NOTES:	CONSTRUCTION NOTES
SHT. PLAN 1:	MAIN LEVEL & BALCONY LAYOUT and FRAMING PLAN
	AND FOUNDATION LAYOUT and FLOOR FRAMING PLAN
SHT. PLAN 2:	ROOF SYSTEM & ATTIC LAYOUT and FRAMING PLANS
SHT. ELEVATION 1:	FRONT ELEVATION
SHT. ELEVATION 2:	NORTH SIDE ELEVATION
SHT. ELEVATION 3:	REAR ELEVATION
SHT. ELEVATION 4:	SOUTH SIDE ELEVATION
SHT. DETAILS 1:	CELLAR AREA: "L"-WALL RETAINING WALL DETAIL
SHT. DETAILS 2:	STRUCTURAL REPAIR & REINFORCEMENT DETAILS
SHT. DETAILS 3:	STRUCTURAL REPAIR & REINFORCEMENT DETAILS

CONSTRUCTION NOTES:

C. CONCRETE:

CONCRETE USED FOR THIS PROJECT SHALL MEET OR EXCEED THE FOLLOWING COMPRESSIVE STRENGTH AT 28 DAYS:

FOUNDATION FOOTINGS & WALLS 3000 psi NORMAL WT. 3000 psi NORMAL WT. SLAB-ON-GRADE (INTERIOR) 3500 psi NORMAL WT. SLAB-ON-GRADE (EXTERIOR)

3000 psi NORMAL WT. **ELEVATED SLABS** MASONRY GROUT 2000 psi NORMAL WT. 2000 psi NORMAL WT. BOND BEAM GROUT

2. ALL CONCRETE SUBJECT TO FREEZE/THAW CYCLES SHALL BE AIR ENTRAINED.

3. FOOTINGS MAY BE PLACED DIRECTLY AGAINST THE SOIL WITH ADEQUATE FORMING.
4. CONCRETE SLABS SHALL BE POURED IN A STRIP PATTERN. CONSTRUCTION JOINTS SHALL COINCIDE WITH THE CONTROL JOINTS (INDICATED AS C.J. ON THE PLANS). CONSTRUCTION JOINTS IN THE SLAB SHALL HAVE THE REINFORCING CONTINUE THROUGH THE JOINT U.N.O. IN THE DETAILS.

5. CONTROL JOINTS AND WATER-STOPS SHALL BE INSTALLED, AS NEEDED, IN ACCORDANCE WITH THE BUILDING CODE REQUIREMENTS, COMMON CONSTRUCTION PRACTICES AND THESE CONSTRUCTION DOCUMENTS.

6. ALL CONTROL JOINTS MAY BE SAW-CUT OR CONSTRUCTED WITH A METAL CONTROL JOINT FORM. CONTROL JOINTS SHALL BE SEALED WITH A SUITABLE JOINT SEALANT.

7. SAW-CUT OR TOOLED CONTROL JOINTS SHALL BE AS DETAILED. SAW-CUTS SHALL BE MADE AS SOON AS PRACTICAL WITHOUT DAMAGING THE FINISHED CONCRETE FLOOR SURFACE (WITHIN 12 HOURS, MAX.)

8. A DIAMOND PATTERN CONTROL JOINT SHALL BE PROVIDED AT ALL ISOLATED COLUMN LOCATIONS, AS INDICATED ON THESE PLANS. THE CORNERS OF THE DIAMOND SHALL COINCIDE WITH THE CONTROL JOINTS, AS SPECIFIED ON THESE PLANS AND BE FILLED WITH 1/2" PREFORMED JOINT FILLER.

9. EXPOSED CONCRETE EDGES SHALL BE BUILT SQUARE AND RUBBED TO A MINIMUM RADIUS CHAMFER, UNLESS SPECIFIC

CHAMFERS ARE CALLED FOR IN DETAILS.

10. CONCRETE FORMWORK SHALL BE DESIGNED BY A REGISTERED PROFESSIONAL ENGINEER LICENSED IN THE COMMONWEALTH OF VIRGINIA.

11. ALL CONCRETE WALLS SHALL BE CONSTRUCTED TO THE NOMINAL DIMENSIONS AS INDICATED ON THE CONSTRUCTION PLANS. THE CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 3000 psi AT 28 DAYS AND PLACED WITH A MAXIMUM SLUMP OF 4". ALL WORK SHALL CONFORM TO THE STANDARDS AND SPECIFICATIONS OF THE AMERICAN CONCRETE

12. ALL REINFORCING STEEL SHALL COMPLY WITH ASTM A615, GRADE 60 STEEL. MINIMUM COVER OVER THE STEEL SHALL BE 2" FOR ALL FOUNDATION WALL CONSTRUCTION. ALL REINFORCING STEEL FOR THE WALLS, HEADERS, GIRDERS, ETC. SHALL BE IN PLACE PRIOR TO THE POURING OF THE WALLS. THE REINFORCING STEEL SHALL BE PROPERLY TIED AND LOCATED IN ACCORDANCE WITH THE DESIGN PLANS.

13. INTER-LOCKING METAL FORMS SHALL BE USED FOR THE CONSTRUCTION OF THE CONCRETE WALL SYSTEM. ALL FORMS SHALL BE CLEAN, PLUM, SQUARE AND PROPERLY ALIGNED FOR CONCENTRIC LOADING ON THE WALL FOOTINGS. ALL DOOR AND WINDOW ELEMENTS SHALL BE BOXED-FRAMED INSIDE OF THE FORMWORK TO THE APPROPRIATE HORIZONTAL AND VERTICAL DIMENSIONS WITH PROPER ALLOWANCES MADE FOR THE FRAMEWORK OF THE DOOR AND WINDOW UNITS. THE FORM SYSTEM SHALL BE PROPERLY PREPPED SO AS TO PREVENT ADHESION OF THE CONCRETE TO THE FORMWORK DURING THE CURING PROCESS

14. ALL THROUGH-WALL PENETRATIONS SHALL BE IN PLACE PRIOR TO THE POURING OPERATION AND PRECAUTIONS SHALL BE TAKEN TO PREVENT FUTURE LEAKAGE IN THE AREAS OF THESE PENETRATIONS (i.e. WATER-PROOF EXPANSIVE FILLER MATERIAL SHALL BE INJECTED INTO THE VOID AREAS OF THE PENETRATION).

15. SEE ARCHITECTURAL, BUILDING, MECHANICAL AND PLUMBING DRAWINGS FOR LOCATIONS OF OPENINGS AND SLEEVES. DO NOT CUT REINFORCEMENT.

D REINFORCEMENT

ALL REINFORCING STEEL SHALL BE DEFORMED BARS IN ACCORDANCE WITH ASTM A615 SPECIFICATIONS, GRADE 60.

2. WELDED WIRE FABRIC REINFORCING SHALL CONFORM TO ASTM A185. THE WELDED WIRE FABRIC SHALL HAVE A MINIMUM STEEL YIELD STRENGTH OF 65,000 psi. (Fy = 65,000 psi).

3. MAINTAIN MINIMUM CONCRETE COVERAGE FOR REINFORCING AS SPECIFIED IN ACI-318.

CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH:

CONCRETE EXPOSED TO EARTH OR WEATHER:

2" (NO. 6 THROUGH NUM. 8 BARS)

1-1/2" (NO. 5 BAR, W31 OR D31 WIRE OR SMALLER)

CONCRETE NOT EXPOSED TO WEATHER OR IN

CONTACT w/ GROUND - SLABS, WALLS & JOISTS: 3/4" (NO. 11 BARS & SMALLER)

4. ALL CONTINUOUS REINFORCING IN FOOTINGS SHALL BE LAP-SPLICED 2'-0". ALL OTHER SPLICES SHALL BE CLASS B TENSION, 32" LAP (MIN.), U.N.O.

5. DETAILING, FABRICATION, AND INSTALLATION OF REINFORCING BARS SHALL COMPLY WITH THE "DESIGN HANDBOOK OF THE CRSI" AND THE MANUAL OF STANDARD PRACTICE OF THE ACI.

6. PROVIDE DOWELS TO MATCH REINFORCEMENT IN ALL WALLS, PIERS, COLUMNS, AND FOUNDATIONS.

7. ALL PEDESTALS OR HOLES IN THE CONCRETE SLAB SHALL HAVE, ADDITIONALLY, (2) #4 x (OPENING DIM. + 2 FT.) ALONG WEALTH OF EACH SIDE OF OPENING AND (2) #3 x 2'-0" DIAGONALLY AT EACH PEDESTAL CORNER.

E. MASONRY:

1. ALL MASONRY CONSTRUCTION SHALL CONFORM TO THE LATEST EDITION OF THE STANDARDS AND SPECIFICATIONS OF THE MASONRY STANDARDS JOINT COMMITTEE.

2. CONCRETE MASONRY UNITS (CMU) SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 1500 psi @ 28 DAYS.

3. THE MINIMUM CMU PRISM STRENGTH SHALL BE 1900 psi. (F'm=1900 psi).

4. THE BLOCK MORTAR SHALL BE TYPE "M" MORTAR FOR ALL MASONRY MATERIALS BELOW GRADE AND ALL OTHER MORTAR SHALL BE TYPE "S" IN ACCORDANCE WITH ASTM C270.

5. THE CMU SHALL BE REINFORCED WITH STEEL REINFORCING BARS THAT COMPLY WITH ASTM A615 GRADE 60 STEEL. TYPICALLY, THE HORIZONTAL JOINT REINFORCEMENT OF THE CMU SHALL CONSIST OF TRUSS TYPE WELDED WIRE, 9 GAUGE, PLACED IN EVERY OTHER COURSE. MORTAR JOINTS SHALL BE UNIFORM ? THICK JOINTS AS REQUIRED FOR COURSING.

6. FULLY GROUT AND REINFORCE, AS SPECIFIED, ALL POINTS OF BEARING IN THE CMU WALLS. ALL GROUT SHALL HAVE A MINIMUM COMPRESSION STRENGTH OF 2000 psi AT 28 DAYS.

7. IF APPLICABLE, ALL MASONRY BOND BEAMS SHALL BE GROUTED SOLID WITH 2000 psi CONCRETE GROUT.

8. ALL MASONRY BELOW GRADE SHALL BE GROUTED SOLID AT POINTS OF BEARING AND WHERE REINFORCED.

9. SOLID MASONRY PIERS, IDENTIFIED AS SMP ON PLAN, SHALL BE GROUTED SOLID FULL HEIGHT OF WALL TO FOUNDATION

1. THE CONTRACTOR SHALL PROVIDE ALL WOOD NAILS, BOLTS, SCREWS, FRAMING ANCHORS, CONSTRUCTION ADHESIVES, ETC., AS NEEDED, TO PERFORM THE ROUGH AND FINISH CARPENTRY FOR THIS PROJECT.

2. ALL INTERIOR FRAMING DIMENSIONS ARE FROM FACE TO FACE OF THE ACTUAL FRAMING LUMBER

3. ALL FRAME WALLS SHALL BE CONSTRUCTED WITH 2x4 OR 2x6 NOMINAL WOOD STUDS, AS INDICATED ON THE PLANS, WITH A DOUBLE TOP PLATE AND SINGLE BOTTOM PLATE OR AS SPECIFIED ON THE PLANS. THE SPACING OF THE WOOD STUD MEMBERS SHALL BE AS SPECIFIED ON THE PLANS. ALL WOOD MATERIALS WHICH ARE IN CONTACT WITH CONCRETE, MASONRY, STEEL, EARTH, ETC. SHALL BE A WOOD PRESERVATIVE, PRESSURE TREATED MATERIAL. THE TREATMENT SHALL BE IN ACCORDANCE WITH AWPA TREATMENT U1, USING A WATERBORNE PRESERVATIVE WITH 0.25 PERCENT TYPICAL RETAINAGE OR A 0.40 PERCENTAGE RETAINAGE FOR MATERIAL IN CONTACT WITH SOIL.

4. UNLESS NOTED OTHERWISE, ALL WOOD STUD FRAMING, JOIST AND RAFTER MEMBERS SHALL BE SPRUCE-PINE-FIR, #2 GRADE (SPF #2) MATERIAL, OR EQUAL, AND SHALL BE FASTENED IN ACCORDANCE WITH THE INTERNATIONAL BUILDING CODE (2012), OR AS SPECIFIED ON THESE DRAWINGS. ALL SPRUCE-PINE-FIR FRAMING LUMBER SHALL HAVE A MINIMUM BENDING STRESS (Fb) OF 875 psi, A MINIMUM HORIZONTAL SHEAR STRESS (Fv) OF 135 psi, A MINIMUM COMPRESSIVE STRENGTH PERPENDICULAR TO THE GRAIN (Fc) OF 425 psi AND A MINIMUM MODULUS OF ELASTICITY (E) OF 1400 ksi. SOLID WOOD BLOCKING SHALL BE INSTALLED WHERE REQUIRED FOR STRUCTURAL BRACING, FIRE STOPPING AND DRAFT STOPPING.

5. UNLESS NOTED OTHERWISE, ALL TRUSS MEMBERS SHALL BE SOUTHERN YELLOW PINE #2 GRADE (SYP #2) MATERIAL, OR EQUAL. ALL SOUTHERN YELLOW PINE FRAMING LUMBER SHALL HAVE A MINIMUM BENDING STRESS (Fb) OF 975 psi, A MINIMUM HORIZONTAL SHEAR STRESS (Fv) OF 175 psi, A MINIMUM COMPRESSIVE STRENGTH PERPENDICULAR TO THE GRAIN Z (Fc) OF 565 psi AND A MINIMUM MODULUS OF ELASTICITY (E) OF 1600 ksi.

6. ALL MICRO=LAM, PARALLAM & PRE-MANUFACTURED TRUSSES, SHALL BE HANDLED, STORED, CUT, AND INSTALLED IN ACCORDANCE WITH THE STANDARDS AND AND SPECIFICATIONS OF THE MANUFACTURER.

7. ALL MICRO=LAM JOIST AND BEAM ELEMENTS SHALL HAVE AN ALLOWABLE BENDING STRESS (Fb) OF 2600 psi, A HORIZONTAL SHEAR STRESS (Fv) OF 285 psi, A COMPRESSIVE STRENGTH PERPENDICULAR TO THE GRAIN (Fc) OF 750 psi AND A MODULUS OF ELASTICITY (E) OF 1900 ksi.

8. FLOOR SHEATHING SHALL BE 23/32" (3/4" NOMINAL) APA RATED STURD-I-FLOOR, OR EQUIVALENT, TONGUE & GROOVE, PLYWOOD, PANELS SHALL HAVE THE LONG DIMENSION ORIENTED ACROSS THREE OR MORE JOISTS AND SHALL BE FASTENED WITH CONSTRUCTION ADHESIVE AND 10d NAILS AT 6" o.c. AT ALL PANEL EDGES AND AT 12" o.c. AT ALL INTERMEDIATE SUPPORTS. UNLESS NOTED OTHERWISE, PANEL EDGES ARE NOT REQUIRED TO BE BLOCKED.

9. EXTERIOR WALLS WITH STUCCO FINISH SHALL BE SHEATHED WITH 23/32" (3/4" NOMINAL) APA RATED EXTERIOR-GRADE SHEATHING, EXTERIOR WALLS WITHOUT STUCCO FINISH SHALL BE SHEATHED WITH THE MINIMUM THICKNESS AS INDICATED ON THE SHEAR WALL SCHEDULE ON THE PLANS. EXTERIOR WALL SHEATHING SHALL BE FASTENED TO WOOD FRAMING AS INDICATED ON THE SHEAR WALL SCHEDULE. UNLESS NOTED OTHERWISE, PANEL EDGES ARE NOT REQUIRED TO BE

10. ROOF DIAPHRAGMS WITH SUPPORT MEMBERS SPACED AT 16" o.c. OR LESS SHALL BE SHEATHED WITH 15/32" (1/2" NOMINAL) APA RATED EXTERIOR-GRADE SHEATHING, WITH A SPAN RATING OF 24/0 OR BETTER, ROOF DIAPHRAGMS WITH SUPPORT MEMBERS SPACED AT GREATER THAN 16" o.c. SHALL BE 19/32" (5/8" NOMINAL) APA EXTERIOR-GRADE SHEATHING, FASTEN PANELS TO WOOD FRAMING WITH 8d NAILS AT 6" o.c. AT ALL PANEL EDGES AND 12" o.c. AT ALL INTERMEDIATE SUPPORTS. PANELS SHALL HAVE THE LONG DIMENSION ORIENTED ACROSS THREE OR MORE SUPPORT MEMBERS. UNLESS NOTED OTHERWISE, PANEL EDGES ARE NOT REQUIRED TO BE BLOCKED.

11. BUILDING PAPER SHALL BE NO. 30 ASPHALT FELT OR SPUN BONDED POLYETHYLENE. THE AIR INFILTRATION BARRIER SHALL BE TYVEK, OR EQUAL.

12. UNLESS NOTED OTHERWISE, ALL ANCHOR BOLTS SHALL BE & ø, ALL CARRIAGE BOLTS SHALL BE & ø AND ALL EXPANSION BOLTS SHALL BE 3" Ø.

G. UTILITY SYSTEMS:

TIMOTHY G. PAINTER 5

Lic. No. 018260

SSIONAL ENGI ONAL EN IN 1. THE PLUMBING, MECHANICAL, AND ELECTRICAL SYSTEMS ARE BEYOND THE SCOPE OF SERVICES OF THESE PLANS. SEPARATE DESIGNS AND CONSTRUCTION DRAWINGS SHALL BE DONE BY OTHERS AND SUBMITTED TO THE BUILDING DEPARTMENT FOR REVIEW AND PERMITS.

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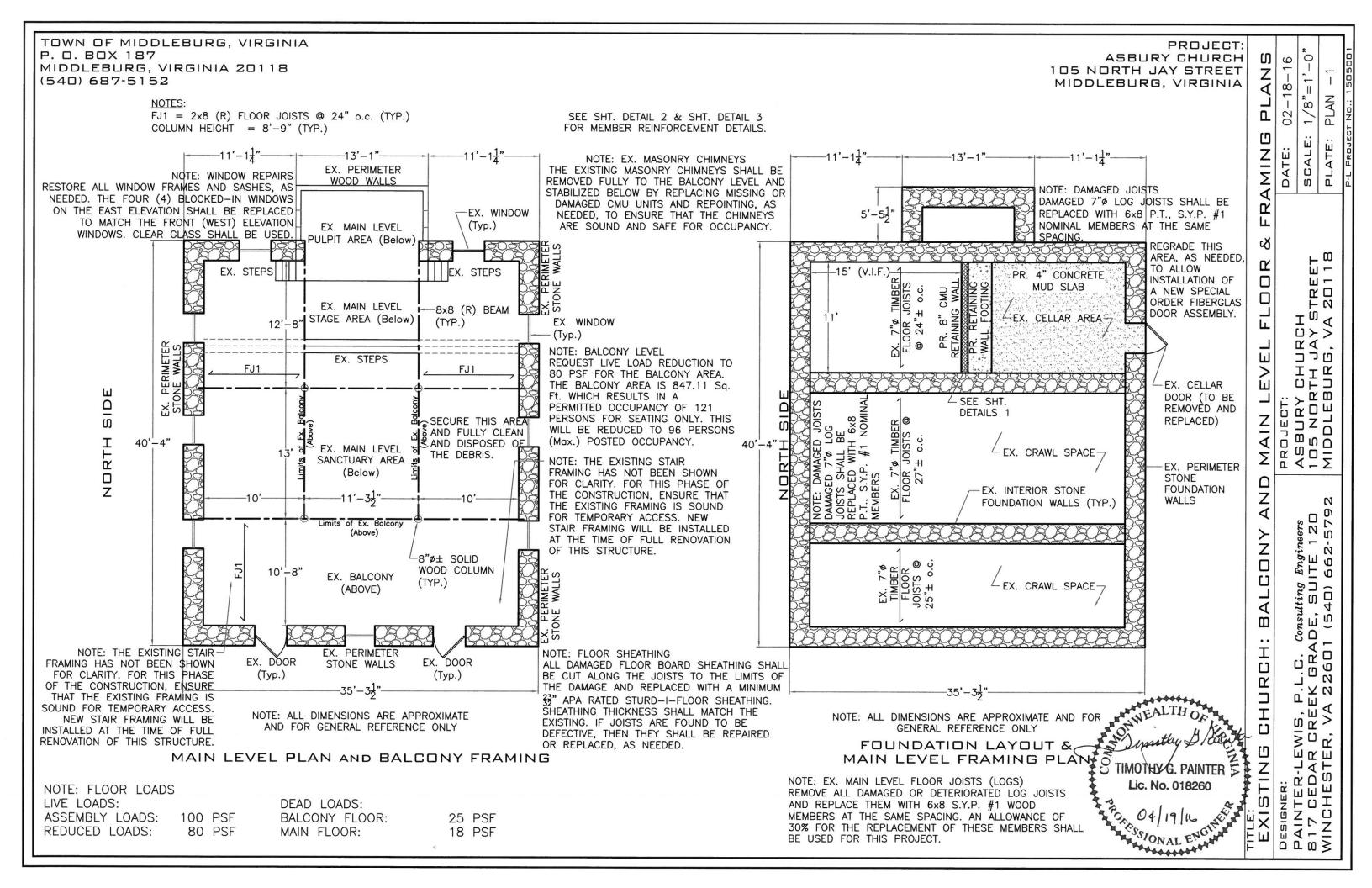
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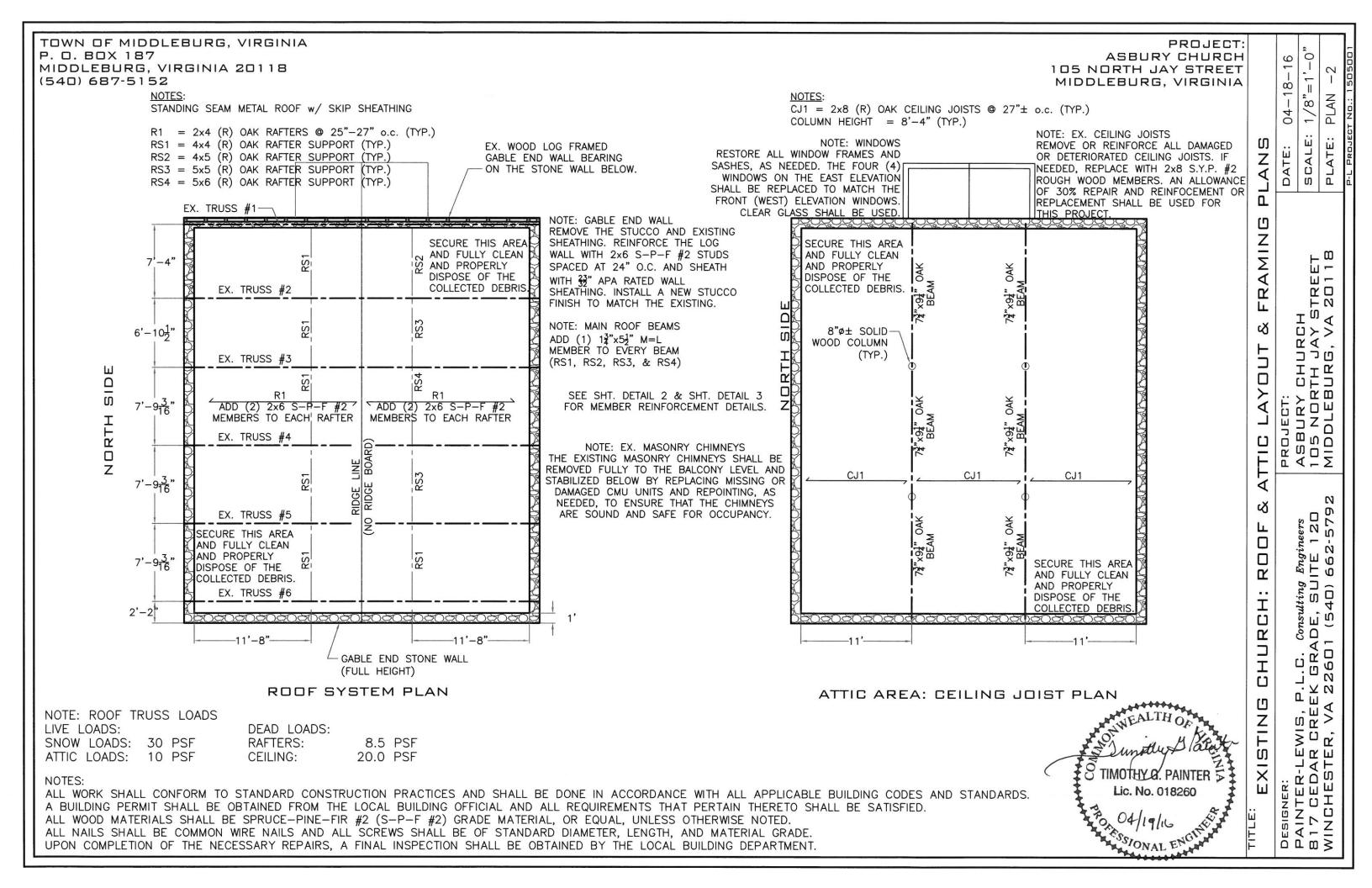
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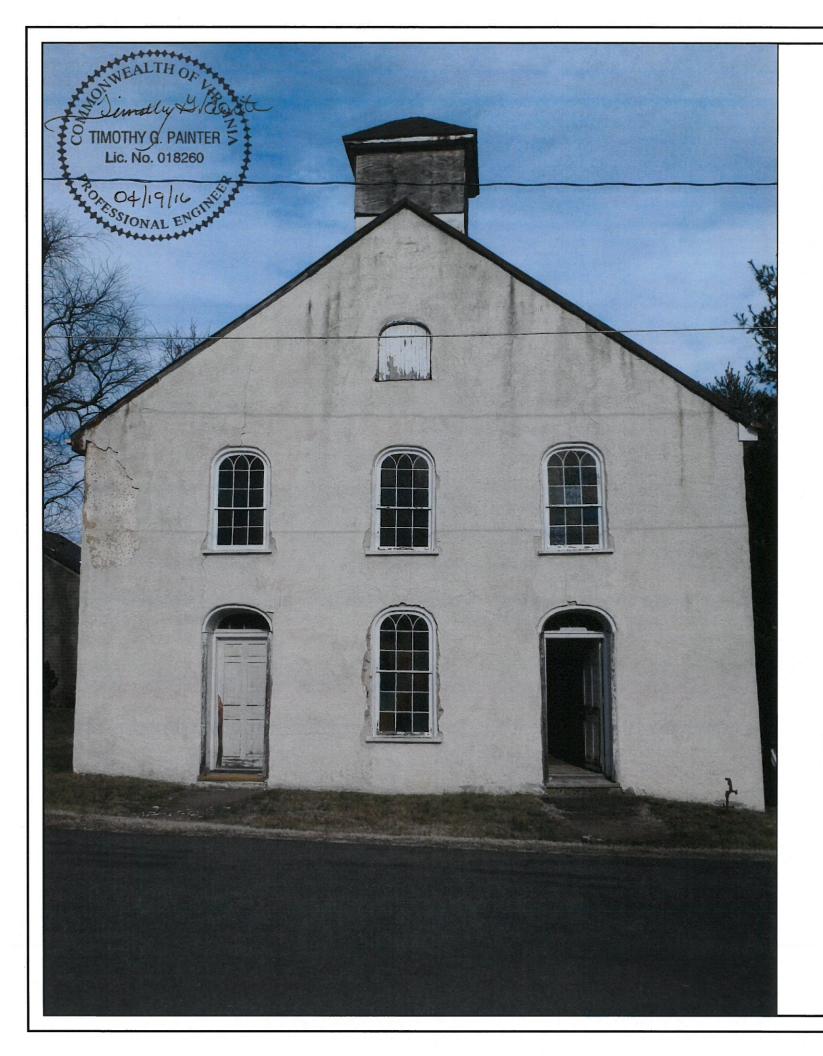
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REMOVE AND REPLACE THE EX. ROOFING SYSTEM OF THE STEEPLE.

REMOVE AND REPLACE THE PLYWOOD OVER THE VENTS OF THE STEEPLE AND INSTALL MATCHING WOOD VENTS WITH THE APPROPRIATE FLASHING AND PROTECTION FROM THE WEATHER.

INSTALL EAVE EXTENSIONS AND FASCIA.

ENSURE THAT THE WINDOWS ARE SECURE AND RESTORE THE WINDOWS FOR USE. REPAIR THE FRAMES AND SASHES, AS NEEDED, TO PROTECT THE WINDOWS (TYPICAL OF ALL). REPLACE THE FOUR (4) PLYWOOD COVERED WINDOWS ON THE EASTERN ELEVATION WITH NEW WINDOWS THAT MATCH THE FRONT (WESTERN) ELEVATION WINDOWS. THE NEW WINDOWS SHALL MEET THE EXISTING ROUGH OPENING SIZES AND SHALL BE HAVE CLEAR GLASS.

ENSURE THAT THE DOORS ARE FUNCTIONING PROPERLY AND PAINT THEM TO MATCH THE EXISTING COLORS FOR PRESERVATION.

REPAIR THE EAVES, AS NEEDED, AND INSTALL THE NEW FASCIA SYSTEM. REMOVE AND REPLACE THE GUTTER AND DOWNSPOUT SYSTEMS. THE NEW GUTTERS SHALL HAVE A 6" HALF-ROUND SECTION AND THE DOWNSPOUTS SHALL MATCH. A DOWNSPOUT SHALL BE INSTALLED ON EACH END OF THE SIDEWALL ELEVATIONS. A 20' (Min.) LENGTH OF 6"Ø CORRUGATED PLASTIC PIPING SHALL BE INSTALLED AT GRADE TO EACH DOWNSPOUT TO ENSURE THAT THE ROOF RUNOFF IS CONVEYED AWAY FROM THE STRUCTURE.

THE EXISTING OIL TANK BY THE CELLAR DOOR SHALL BE REMOVED AND DISPOSED OF PROPERLY OFFSITE. ANY CONTAMINATED SOILS SHALL BE PROPERLY REMOVED. REGRADE THE AREA FOR POSITIVE DRAINAGE AWAY FROM THE CHURCH.

THE LATERAL GRADING AROUND AND AWAY FROM THE CHURCH SHALL BE A MINIMUM OF 6"(Vertical) OVER 10' (Horizontal). THE GRADING OF THE FRONT YARD AREA SHALL BE A MINIMUM OF 2% SLOPE LONGITUDINALLY TO CONVEY THE STORM WATER RUNOFF TO THE SIDE YARDS.

NOTES: EXTERIOR WALLS & FINISHES

-THE ROOFING SYSTEM SHALL BE FULLY REMOVED AND REPLACED. THE GABLE ENDS SHALL HAVE WOOD BLOCKING INSTALLED ALONG THE ROOF LINES AT 24" ON CENTER. A 2x6 P.T. NAILER SHALL BE CONNECTED TO THE EMBEDDED BLOCKING ALONG THE SLOPE OF THE ROOF. THE PROPOSED 1x FASCIA SHALL BE INSTALLED SO THE GABLE END EAVES ARE BUILT-OUT FOR PROTECTION OF THE WALLS AGAINST WATER INFILTRATION.

-ALL LOOSE AND DAMAGED STUCCO FINISHES SHALL BE REMOVED AND DISPOSED OF PROPERLY.

-ENSURE THAT THE WINDOWS ARE SECURE AND RESTORE THE WINDOWS FOR USE. REPAIR THE FRAMES AND SASHES, AS NEEDED, TO PROTECT THE WINDOWS (TYPICAL OF ALL). REPLACE THE FOUR (4) PLYWOOD COVERED WINDOWS ON THE EASTERN ELEVATION WITH NEW WINDOWS THAT MATCH THE FRONT (WESTERN) ELEVATION WINDOWS. THE NEW WINDOWS SHALL MEET THE EXISTING ROUGH OPENING SIZES AND SHALL BE HAVE CLEAR GLASS.

-MAKE AN ALLOWANCE OF 50% OF THE WALL AREA TO BE REMOVED AND REPAIRED ON EACH ELEVATION. THE REPAIRS SHALL INCLUDE THE REPLACEMENT OF LOOSE OR MISSING STONES IN THE WALL SECTIONS, REPOINTING OF THE STONE MASONRY WALL STRUCTURE WITH A SIMILAR MORTAR TO MATCH THE EXISTING, AND MATCHING THE STUCCO FINISHES, AS NEEDED, TO ENSURE A STABLE AND WATER TIGHT WALL SECTION.

EXISTING CHURCH: FRONT ELEVATION

ELEVATION

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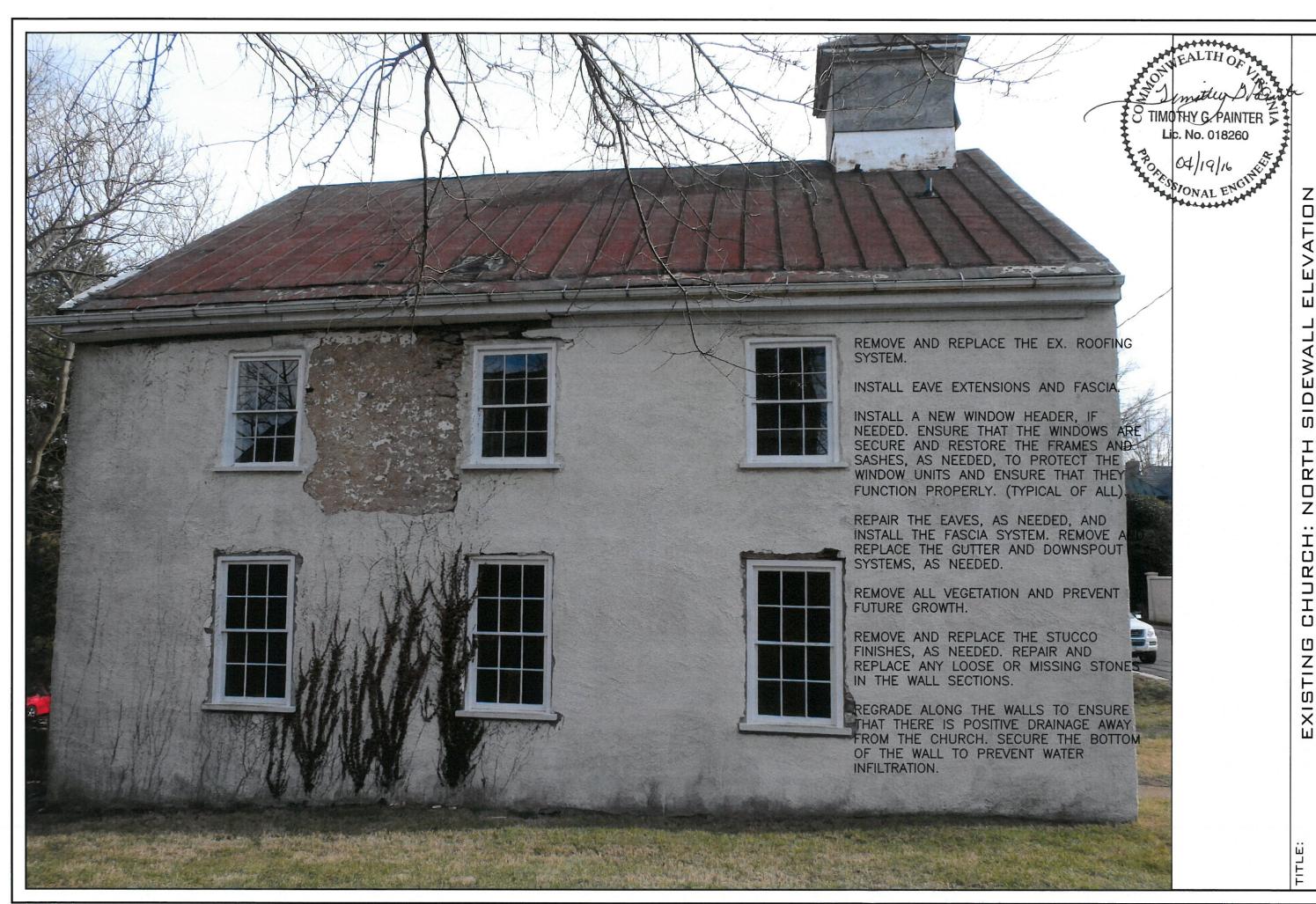
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IEER: FER-LEWIS, P.L.C. *Consulting Engineers* SEDAR GREEK GRADE, SUITE 120 HESTER, VA. 22601 (540) 662-5792

ENGINEER: PAINTER-LEWIS, P.



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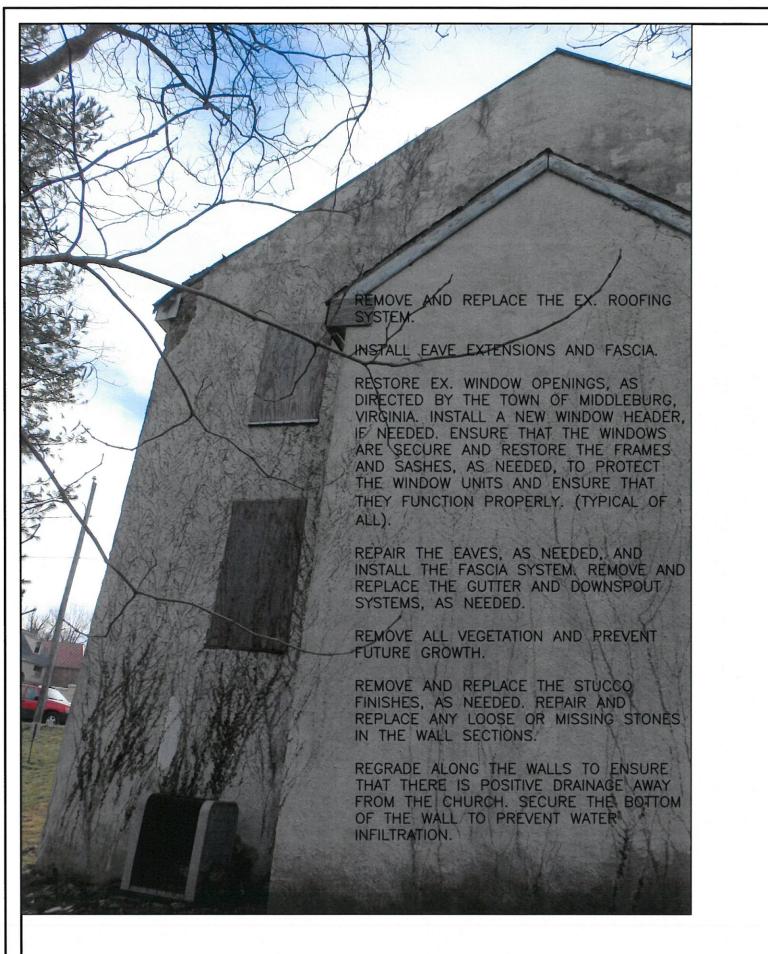
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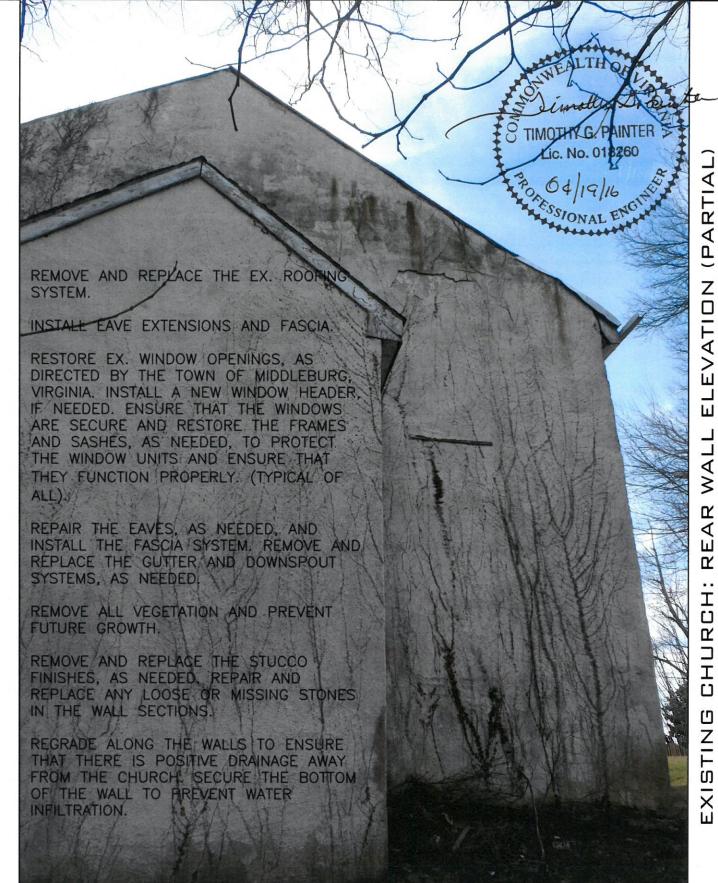
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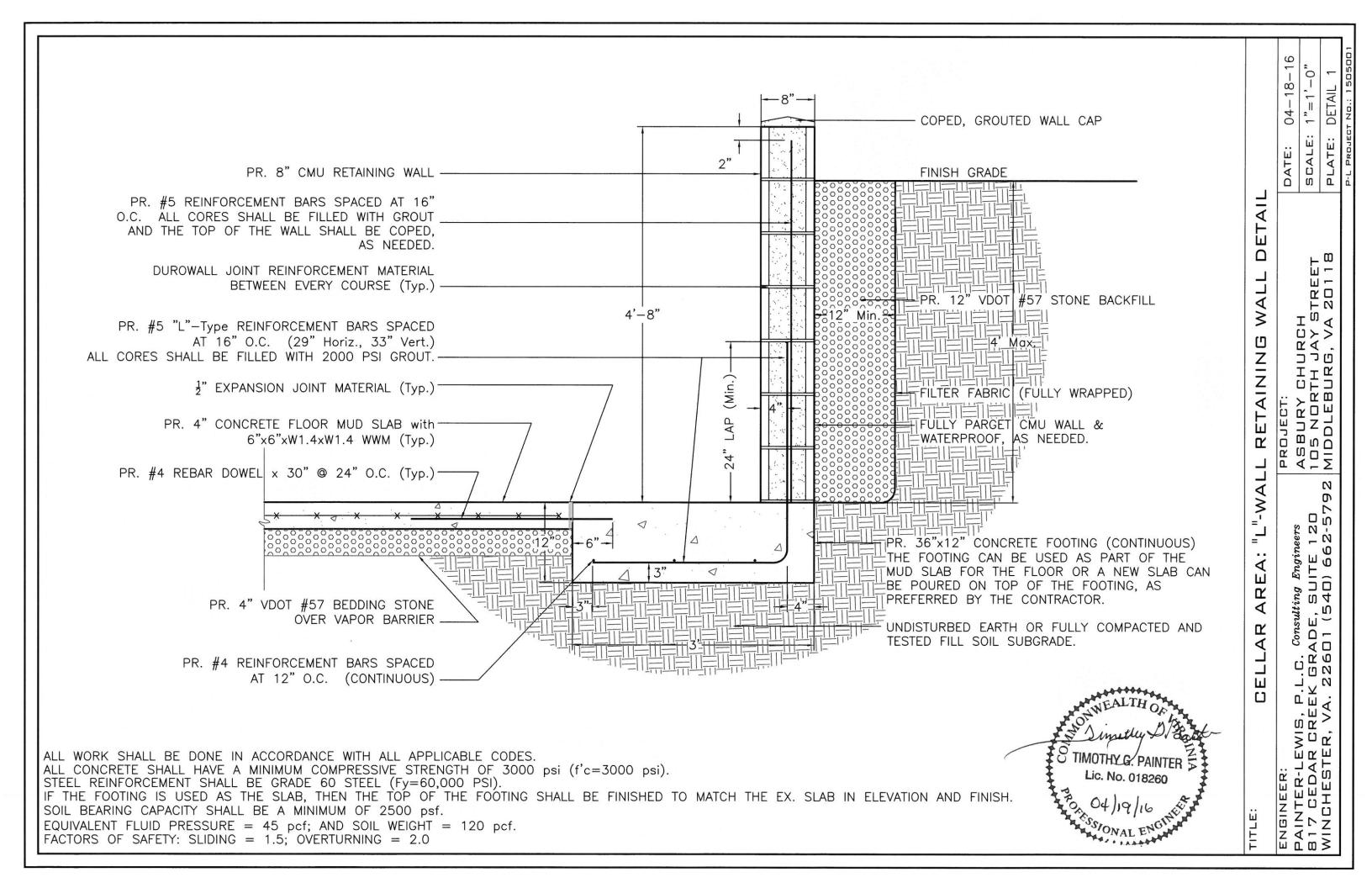
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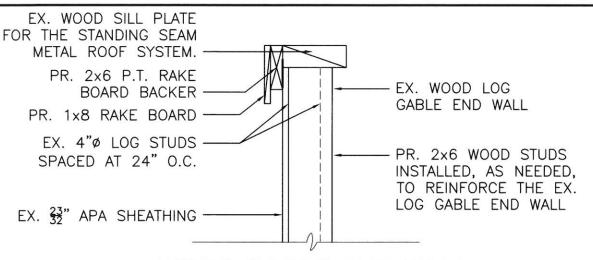
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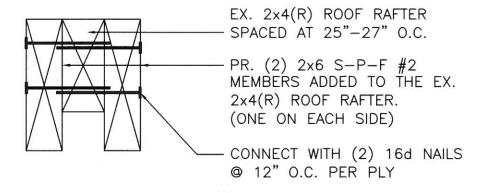
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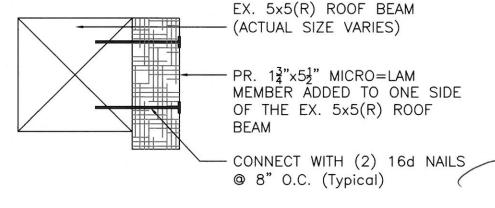
WOOD GABLE END WALL EAVE EXTENSION DETAIL

Scale: 1" = 1'-0"



ROOF RAFTER REINFORCEMENT DETAIL

Scale: 3'' = 1'-0''



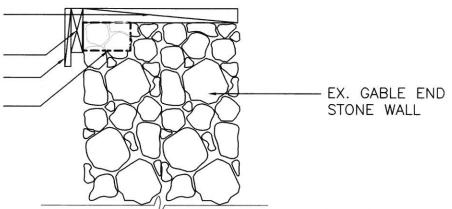
MAIN ROOF BEAM REINFORCEMENT DETAIL

Scale: 3'' = 1'-0''

EX. WOOD SILL PLATE FOR THE STANDING SEAM METAL ROOF SYSTEM. PR. 2x6 P.T. RAKE BOARD BACKER PR. 1x8 RAKE BOARD PR. 4x4 P.T. x6" WOOD NAILERS SPACED AT 24" O.C. EMBEDDED IN THE STONE WALL.

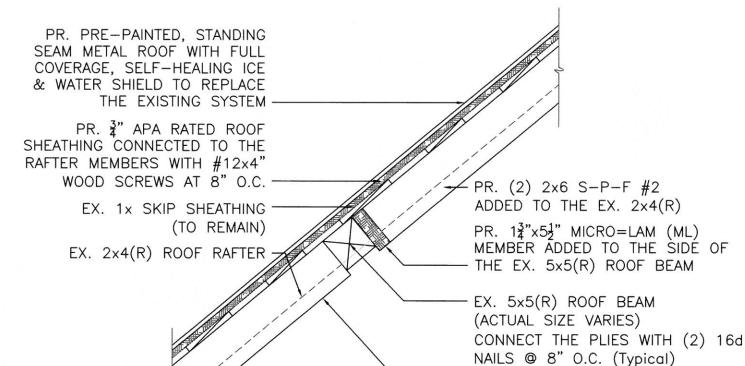
TIMOTHY G. PAINTER

Lic. No. 018260



STONE GABLE END WALL EAVE EXTENSION DETAIL

Scale: 1" = 1'-0"



NOTE: PROPERLY SUPPORT THE EXISTING RAFTERS AND STRUCTURAL COMPONENTS, AS NEEDED, TO INSTALL THE 13/4"x51" ML PLY. CUT THE EX. RAFTERS TO ALLOW INSTALLATION AND INSTALL A SIMPSON L50 ANGLE; THEN INSTALL THE PROPOSED (2) 2x6 S-P-F #2 TO THE EX. 2x4(R) RAFTER. CONNECT

PR. (2) 2x6 S-P-F #2

ADDED TO THE EX. 2x4(R)

MAIN ROOF TYPICAL SECTION

THE NEW PLIES TO THE EXISTING BEAM BY SIMPSON L50 ANGLES.

Scale: 1" = 1'-0"

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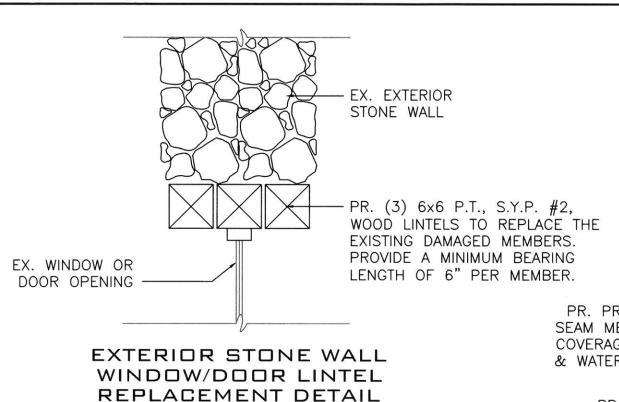
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Engineers 11TE 120 1) 662-57 Consulting Engine ADE, SUITE 01 (540) 66 ADE, 8 .c. GR, 26 Д Н ₹ 0 <u>r</u> ΑΑ ΕΝ. Ω.



EX. 2x(R) MEMBER
(ACTUAL SIZE VARIES)

PR. 2x(R) MEMBER ADDED TO
THE DAMAGED MEMBER (MATCH
EXISTING SIZES AND MATERIAL
TYPES)

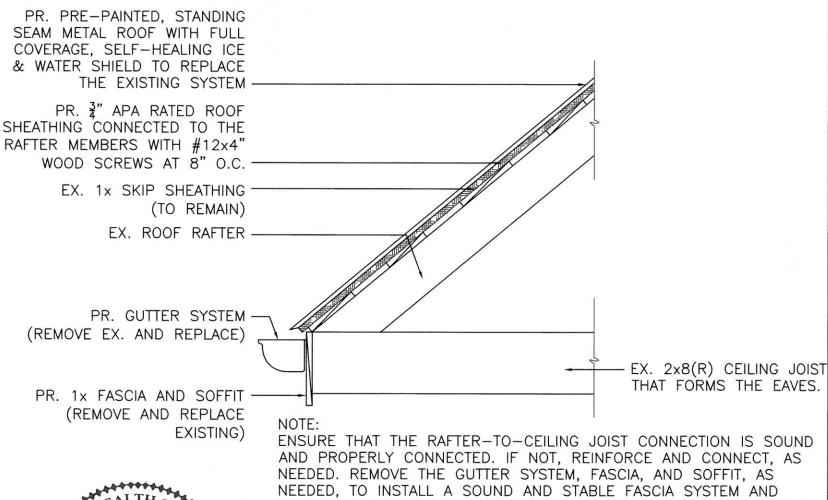
CONNECT WITH (2) 16d NAILS

8" O.C. (Typical)

Scale: 1'' = 1'-0''

TYPICAL DAMAGED MEMBER REINFORCEMENT DETAIL

Scale: 3" = 1'-0"



THE ROOF SYSTEM.

TIMOTHY G. PAINTER

Lic. No. 018260

INSTALL PROPERLY SIZED GUTTERS TO CONTROL THE RUNOFF FROM

MAIN ROOF EAVE

TYPICAL SECTION

Scale: 1" = 1'-0"

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